CONSTRUCTION NOTES

GENERAL NOTES

1. CONFORM TO THE REQUIREMENTS OF THE 2012 ONTARIO BUILDING CODE (OBC) INCLUDING ALL THE LATEST STANDARDS REFERENCED THEREIN, AND ANY APPLICABLE ACTS OF AUTHORITY HAVING JURISDICTION THE LATEST VERSION OF ALL STANDARDS AND CODES LISTED BELOW SHALL BE USED.

READ STRUCTURAL DRAWINGS IN CONJUNCTION WITH ALL OTHER SPECIFICATIONS AND CONTRACT DOCUMENTS.

WHERE DISCREPANCIES EXIST BETWEEN CONTRACT DOCUMENTS, INCLUDING DRAWINGS AND APPLICABLE CODES AND ACTS. THE MOST STRINGENT SHALL GOVERN. CONTRACTOR SHALL CHECK ALL DIMENSIONS ON WORKING DRAWINGS AND REPORT ANY DISCREPANCIES TO THE ENGINEER BEFORE PROCEEDING WITH THE WORK.

4. THESE DESIGN DOCUMENTS ARE PREPARED SOLELY FOR THE USE BY THE PARTY WITH WHOM THE DESIGN PROFESSIONAL HAS ENTERED INTO A CONTRACT AND THERE ARE NO REPRESENTATIONS OF ANY KIND MADE BY THE DESIGN PROFESSIONAL TO ANY PARTY WITH WHOM THE DESIGN PROFESSIONAL HAS NOT ENTERED INTO A CONTRACT.

5. THE USE OF THESE DRAWINGS IS LIMITED TO THAT IDENTIFIED IN THE REVISION COLUMN. DO NOT CONSTRUCT FROM THESE DRAWINGS UNLESS MARKED "ISSUED FOR CONSTRUCTION" BY MTE CONSULTANTS.

6. UNDER NO CIRCUMSTANCES ARE THESE DRAWINGS TO BE SCALED, INCLUDING FOR PREPARATION OF SHOP DRAWINGS, CONSTRUCTION LAYOUT, OR BIDDING PURPOSES. ERRORS MADE BY PERSONS SCALING THESE DRAWINGS SHALL NOT BE THE RESPONSIBILITY OF MTE

7. SEE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATIONS AND SIZES OF PITS. OPENINGS, BASES, HOUSE KEEPING PADS, SUMPS TRENCHES, DEPRESSIONS, GROOVES, CURBS, CHAMFERS AND SLOPES NOT SHOWN ON STRUCTURAL DRAWINGS.

8. BEFORE PROCEEDING WITH WORK, THE CONTRACTOR SHALL VISIT THE SITE AND BECOME FAMILIARIZED WITH ALL CHARACTERISTICS AFFECTING NEW AND EXISTING CONSTRUCTION. ANY CHANGES, ALTERATIONS OR REVISIONS MUST BE REPORTED TO THE ENGINEER BEFORE PROCEEDING WITH THE WORK.

9. SUBSTITUTIONS FROM SPECIFIED PRODUCTS AND MATERIALS MUST BE APPROVED IN WRITING BY THE ENGINEER PRIOR TO ORDERING OF MATERIALS. TH CONTRACTOR SHALL REIMBURSE ALL CONSULTANTS FOR ADDITIONAL COSTS INCURRED AS A RESULT OF REVIEWING ANY CHANGES MADE TO THE CONTRACT

10. ALL WORK IS TO BE PERFORMED IN ACCORDANCE WITH THE OCCUPATIONAL HEALTH AND SAFETY ACT AND REGULATIONS FOR CONSTRUCTION PROJECTS - O.REG.

11. IT IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO DESIGN ALL SHORING AND TEMPORARY BRACING AS PER O.REG 213/91 AND THE CONTRACTOR SHALL RETAIN AN ENGINEER AS REQUIRED.

12. THE CONTRACTOR SHALL RETAIN AN INDEPENDENT INSPECTION AND TESTING COMPANY TO ENSURE THAT ALL WORK IS DONE IN ACCORDANCE WITH THE DRAWINGS AND SPECIFICATIONS. REQUIRED TESTING SHALL BE AS PER THE TESTING AND INSPECTION TABLE.

13. MTE CONSULTANTS WILL PROVIDE GENERAL REVIEW OF CONSTRUCTION IN ACCORDANCE WITH THE PERFORMANCE STANDARDS OF THE ASSOCIATION OF PROFESSIONAL ENGINEERS OF ONTARIO BY MEANS OF A RATIONAL SAMPLING PROCEDURE TO DETERMINE WHETHER THE CONSTRUCTION OF THAT WORK SHOWN ON THE MTE DRAWINGS IS IN GENERAL CONFORMITY WITH THE PLANS, SKETCHES, DRAWINGS, AND SPECIFICATIONS FORMING PART OF THE CONTRACT DOCUMENTS PREPARED BY "MTE". THE CONTRACTOR IS SOLELY RESPONSIBLE FOR QUALITY CONTROL AND THE PERFORMANCE OF THE WORK IN ACCORDANCE WITH THE CONTRACT. "MTE" SHALL NOT BE RESPONSIBLE FOR THE ACTS OR OMISSIONS OF THE CONTRACTOR, SUB-CONTRACTOR, OR ANY OTHER PERSON PERFORMING ANY OF THE WORK OR FOR THE FAILURE OF ANY OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

14. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO NOTIFY THE ENGINEER OF CONSTRUCTION PROGRESS SO THE ENGINEER CAN COMPLETE GENERAL REVIEWS. THE CONTRACTOR SHALL PROVIDE THE ENGINEER WITH A CONSTRUCTION SCHEDULE PRIOR TO STARTING THE WORK. GENERALLY, REVIEWS BY THE ENGINEER WILL BE REQUIRED FOR REBAR PRIOR TO CONCRETE PLACEMENT. FOOTING AND FOUNDATIONS PRIOR TO BACKFILLING, AND ABOVE GRADE FRAMING PRIOR TO INSTALLATION OF INTERIOR FINISHES.

TESTING AND INSPECTION

1. THE FOLLOWING ITEMS REQUIRE TESTING OR INSPECTION BY A CERTIFIED INDEPENDENT TESTING OR INSPECTION AGENCY UNLESS NOTED OTHERWISE. THE AGENCY SHALL SEND COPIES OF ALL STRUCTURAL TESTING AND INSPECTION REPORTS TO THE ENGINEER

| FOR REVIEW. | |
|-----------------------------|----------------------------|
| REQUIRED SUBMITTALS | COMMENTS |
| REINFORCING STEEL PLACEMENT | INSPECT FINAL PLACEMENT |
| STRUCTURAL STEEL WELDING | INSPECT ALL FIELD WELDS |
| MORTAR CUBES | |
| COLD FORMED STEEL | |

CONCRETE AND REINFORCING

1. ALL CONCRETE WORK TO CONFORM TO THE LATEST REQUIREMENTS OF CSA STANDARDS A23.1, A23.2 &

2. REINFORCING BARS SHALL CONFORM TO THE REQUIREMENTS OF CAN/CSA G30.18 GRADE 400R FOR REINFORCING STEEL AND BE DEFORMED HI-BOND HARD GRADE WITH MINIMUM YIELD STRENGTH OF Fy=400MPa.

WELDING OF REINFORCING STEEL SHALL NOT BE PERMITTED UNLESS SPECIFICALLY NOTED ON THE DRAWINGS. IF PERMITTED, GRADE 400W DEFORMED REINFORCING STEEL IS TO BE USED AND WELDING IN ACCORDANCE WITH CSA W186.

4. WELDED WIRE MESH AND WELDED WIRE FABRIC SHALL CONFORM TO THE REQUIREMENTS OF CAN/CSA G30.5 WITH A MINIMUM YIELD STRENGTH OF Fy 450MPa. ALL WELDED WIRE PRODUCTS ARE TO BE SUPPLIED AS FLAT SHEETS AND SHALL BE LAPPED A MINIMUM OF 150mm (6") AT JOINTS (U.N.O.).

5. DETAILING AND PLACING OF ALL REINFORCING STEEL SHALL BE IN ACCORDANCE WITH THE REINFORCING STEEL INSTITUTE OF CANADA "MANUAL OF STANDARD

6. ALL REINFORCING STEEL SHALL BE SHOP FABRICATED TO INCLUDE HOOKS AND BENDS AS

PRACTICE".

(U.N.O.).

7. ALL REINFORCING LAP SPLICES SHALL CONFORM TO THE LATEST CSA STANDARD A23.3 AND ALL BAR SPLICES SHALL BE CLASS "B" TENSION SPLICES

7.1. NO BAR SPLICES SHALL BE LESS THAN IN THE 7.2. INCREASE HORIZONTAL SPLICE LENGTHS IN THE TABLE BY 1.3 WHERE MORE THAN 300mm (12") OF FRESH CONCRETE IS CAST BELOW THE SPLICE

| OF TRESH CONCRETE IS GREET BELOW THE SHELOE. | | | | | |
|--|-------|-------------|-------|--------|--|
| BAR | TENSI | COMPRESSION | | | |
| SIZE | 25MPa | 30MPa | 35MPa | SPLICE | |
| | CONC. | CONC. | CONC. | (mm) | |
| 10M | 400 | 400 | 400 | 450 | |
| | (16") | (16") | (16") | (18") | |
| 15M | 600 | 600 | 600 | 450 | |
| | (24") | (24") | (24") | (18") | |
| 20M | 800 | 800 | 800 | 600 | |
| | (32") | (32") | (32") | (24") | |
| 25M | 1200 | 1100 | 1000 | 750 | |
| | (48") | (44") | (40") | (30") | |
| | 1400 | 1300 | 1200 | 900 | |

(56") (52") (48") (36")

1400

(56")

1050

(42")

8. ALL DOWEL EMBEDMENT SHALL MATCH THE ABOVE TENSION SPLICE LENGTH, UNLESS NOTED OTHERWISE. ALL HORIZONTAL BARS SHALL BE HOOKED 300mm

1500

(66")

(66")

(12") AROUND CORNERS.

DELETERIOUS MATERIALS.

JOB SITE.

10. ALL REINFORCING STEEL FABRICATION AND PLACEMENT DRAWINGS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW BEFORE FABRICATION.

SUPPORTS AND SYMMETRICALLY IN SPANS, UNLESS NOTED OTHERWISE. 12. REINFORCING BARS, DOWELS AND ANCHOR BOLTS SHALL BE SECURELY TIED IN PLACE SO AS TO MAINTAIN THEIR EXACT POSITION BEFORE AND DURING

PLACEMENT OF CONCRETE, BAR SUPPORTS SHALL ONLY

11. PLACE REINFORCING BARS SYMMETRICALLY OVER

BE MADE OF PRECAST CONCRETE BLOCKS, PLASTIC OR 13. ALL OIL, GREASE, MUD AND DEBRIS SHALL BE ENTIRELY REMOVED FROM THE REINFORCING STEEL AND ANCHOR BOLTS PRIOR TO THE PLACEMENT OF CONCRETE. REBAR SHALL BE STORED ON SITE IN A MANNER TO BE KEPT CLEAN AND FREE FROM

14. CONFORM TO THE CONCRETE COVER REQUIREMENTS OF CSA A23.1 AND THE FOLLOWING, UNLESS NOTED 14.1. CONCRETE CAST AGAINST EARTH: 75mm (3") 14.2. PIERS AND WALL: 40mm (1½") 14.3. EXPOSED TO DE-ICING CHEMICALS: 60mm (21/2") 14.4. INTERIOR SLABS AND BEAMS: 40mm (1½")

15. CONCRETE PROPERTIES: 15.1. ALL CONCRETE SHALL HAVE A 28 DAY MINIMUM COMPRESSIVE STRENGTH OF 20MPa UNLESS OTHERWISE SPECIFIED. 15.2. CONCRETE MIX DESIGN SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO USE AT

| USE | CSA CLASS | 28 DAY COMP. STRENGTH (MPa) | MAX. W/C RATIO | AIR CONTENT (%) | MAX. AGGREGATE SIZE (mm) | SLUMP (mm) |
|-------------------------------|-----------|--------------------------------|----------------|-----------------|-----------------------------|------------|
| OUNDATION/ RETAINING WALLS | F-2 | 25 | 0.55 | 4-7 | 20 | 80 ±30 |
| | | | | | | |

16. WHEN SUPER-PLASTICIZERS ARE USED. THE SLUMP MAY BE INCREASED BEYOND THE VALUES GIVEN, BUT SHALL BE BELOW THE POINT WHERE SEGREGATION WILL OCCUR. THE COST OF SUPER-PLASTICIZERS SHALL BE INCLUDED IN THE COST OF CONCRETE

17. DO NOT ADD WATER TO CONCRETE UNLESS WRITTEN APPROVAL GIVEN BY THE ENGINEER. IF HIGHER SLUMP CONCRETE IS DESIRED, CONCRETE SUPPLIER SHALL DESIGN AND SUPPLY ACCORDINGLY.

18. HOT AND COLD WEATHER CONCRETING SHALL COMPLY WITH ALL REQUIREMENTS OF CSA STANDARD A23.1. CALCIUM CHLORIDE ADDITIVES WILL NOT BE

19. ALL CONCRETE FORMWORK TOLERANCES AND SURFACE FINISHES SHALL COMPLY WITH CSA STANDARD A23.1 UNLESS NOTED OTHERWISE ON THE ARCHITECTURAL DRAWINGS

20. ALL CONCRETE FORMS TO BE WET THOROUGHLY BEFORE POURING CONCRETE.

21. WATER CURING OF CONCRETE IS RECOMMENDED. CURE AND PROTECT ALL CONCRETE IN ACCORDANCE WITH CSA A23.1 SECTION 7.4.

22. ALL CONCRETE EXCEPT SLABS ON GRADE 150mm (6") THICK OR LESS SHALL BE MECHANICALLY VIBRATED SO AS TO COMPLETELY FILL THE FORM WITHOUT CAUSING UNDUE SEGREGATION. ANY DEFECTS IN THE HARDENED CONCRETE SHALL BE SATISFACTORILY REPAIRED OR SHALL BE REPLACED.

23. CONTROL JOINTS IN SLABS ON GRADE SHALL BE 1/4 THE THICKNESS OF THE SLAB. SPACING OF CONTROL JOINTS IN CONCRETE SLABS-ON-GRADE SHALL NOT EXCEED THE GREATER OF 30 TIMES THE THICKNESS OF THE SLAB OR 4500mm (15'-0") UNLESS NOTED ON

24. WHERE STEEL BEARING PLATES ARE SHOWN ON THE DRAWINGS. THEY SHALL BE ANCHORED WITH A MINIMUM OF TWO 12mm DIA. x 450mm LONG + 50mm (½" DIA. x18" LONG + 2") HOOKED ANCHOR RODS WELDED TO THE PLATES AND EMBEDDED INTO THE CONCRETE.

25. CHECK ALL STRUCTURAL, ARCHITECTURAL MECHANICAL, ELECTRICAL, CIVIL, LANDSCAPE AND ALL OTHER RELEVANT DRAWINGS FOR LOCATIONS AND SIZES OF BOLTS, SLEEVES AND OPENINGS

26. SUPPLY AND SET ANCHOR BOLTS, SLEEVES, PIPE HANGERS, JOISTS AND OTHER INSERTS AND OPENINGS AS INDICATED OR SPECIFIED ELSEWHERE. 26.1. FOR BEAMS AND COLUMNS: NO SLEEVES, DUCTS. PIPES OR OTHER OPENINGS SHALL PASS VERTICALLY OR HORIZONTALLY EXCEPT WHERE EXPRESSLY DETAILED ON STRUCTURAL DRAWINGS

OR WHERE APPROVED IN ADVANCE BY ENGINEER. 26.2. FOR SLABS AND WALLS: ALL SLEEVES AND OPENINGS GREATER THAN 100mm (4") IN ANY DIMENSION OR REQUIRING THE CUTTING OF ANY REINFORCEMENT, AND NOT INDICATED ON STRUCTURAL DRAWINGS, MUST BE APPROVED BY THE ENGINEER

26.3. FOR MULTIPLE OPENINGS OR SLEEVES: IF WITHIN 600mm (24") OF EACH OTHER CONSULT ENGINEER FOR DIRECTION.

27. CAST IN ANCHOR BOLTS SHALL CONFORM TO THE LATEST CSA STANDARD G40.21 OR ASTM F1554 WITH A MINIMUM YIELD STRENGTH OF 250MPa AND SHALL BE SET TRUE AS TO LOCATION, ELEVATION AND PROJECTION TO THE FOLLOWING TOLERANCES: 27.1. ANCHOR BOLT LOCATION = ± -3 mm ($\frac{1}{2}$ "). 27.2. ANCHOR BOLT PROJECTION = +/-6mm ($\frac{1}{4}$ ").

28. UNLESS NOTED OTHERWISE ON DRAWINGS ALL REFERENCES TO EPOXY ARE FOR HILTI HIT-HY 200. FOLLOW MANUFACTURER'S RECOMMENDED INSTALLATION

1. ALL MASONRY CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS OF CSA STANDARDS CAN/CSA-A370, CAN/CSA-A371 AND CSA S304.1.

MASONRY

2. ALL MASONRY UNITS OF CONCRETE SHALL CONFORM TO THE CSA STANDARD CAN/CSA-A165 AND SHALL HAVE A MINIMUM LOAD BEARING STRENGTH OF 15MPa BASED ON NET CROSS-SECTIONAL AREA.

3. REINFORCING BARS SHALL CONFORM TO THE REQUIREMENTS OF CAN/CSA G30.18 GRADE 400R FOR REINFORCING STEEL AND BE DEFORMED HI-BOND HARD GRADE WITH MINIMUM YIELD STRENGTH OF Fy = 400MPa.

4. TYPE S MORTAR SHALL BE USED THROUGHOUT FOR

LOAD BEARING BLOCK. TYPE N MORTAR SHALL BE USED FOR BRICK VENEER OR DECORATIVE NON-LOAD BEARING 4.1. MORTAR CUBE COMPRESSIVE STRENGTHS FROM

MORTAR TYPE | JOB PREPARED | LAB PREPARED 8.5MPa 12.5MPa

5.0MPa

4.2. MORTAR MIX PROPORTIONS: MIX ACCORDING TO TABLE 3 OR 4 OF CSA A179. MORTAR MIX SHALL BE TESTED FOR STRENGTH AND APPROVED BY THE ENGINEER PRIOR TO USE ON THE JOB.

4.3. GROUT: WHERE CALLED FOR ON DRAWINGS SHALL

CONFORM TO CAN/CSA A179 MINIMUM 28 DAY

3.5MPa

5. ALL MASONRY WALLS SHALL BE HORIZONTALLY REINFORCED WITH NO.9 (3.7mm) CONTINUOUS JOINT REINFORCING AT EVERY SECOND COURSE, 400mm (16"). USE STANDARD LADDER TYPE REINFORCING FOR GROUTED AND REINFORCED WALLS, TRUSS TYPE FOR UNREINFORCED WALLS.

STRENGTH 20MPa.

5.1. ALL JOINT REINFORCEMENT SHALL BE HOT-DIPPED GAI VANIZED. 5.2. REINFORCEMENT SHALL BE LAPPED A MINIMUM OF 300mm (12") AT ALL JOINTS.

5.3. PREFABRICATED CORNER AND TEE REINFORCEMENT SHALL BE USED AT ALL WALL INTERSECTIONS 5.4. REINFORCEMENT SHALL BE INSTALLED IN THE

FIRST AND SECOND BED JOINTS 200mm (8") APART, BELOW THE TOP OF WALLS. 5.5. REINFORCEMENT SHALL BE INSTALLED IN THE FIRST AND SECOND BED JOINTS 200mm (8") APART, IMMEDIATELY ABOVE LINTELS AND BELOW

SILLS AND SHALL EXTEND 600mm (24") BEYOND THE JAMB. 5.6. REINFORCEMENT SHALL BE PLACED AS TO PROVIDE 16mm (%") MORTAR COVER ON THE EXTERIOR FACE OF WALL AND 12mm (1/2") COVER ON THE INTERIOR FACE OF WALL.

6. ALL TIES FOR MASONRY VENEER SHALL BE DESIGNED AND SUPPLIED BY THE MASONRY CONTRACTOR IN ACCORDANCE WITH CSA STANDARDS S304.1 AND CAN/CSA-A370. ALL TIES TO STEEL STUDS ARE TO BE SIDE MOUNTED (FACE MOUNTED NOT ACCEPTABLE).

7. PROVIDE COLD WEATHER PROTECTION AS REQUIRED BY CAN/CSA-A371 "MASONRY CONSTRUCTION FOR BUILDINGS".

8. ALL BLOCK MASONRY UNITS SHALL BE CONSTRUCTED WITH FULL HEAD JOINTS, AND FULL BED JOINTS UNDER THE FULL BEARING AREAS OF THE FACE SHELLS, AND UNDER WEBS SURROUNDING THOSE CELLS TO BE FILLED WITH GROUT

9. THE INTERSECTION OF ALL MASONRY WALLS SHALL BE TOOTHED OR CONTINUOUSLY REINFORCED WITH JOINT REINFORCEMENT.

O. PROVIDE A MINIMUM DEPTH OF 200mm (8") OF 100% SOLID MASONRY UNITS, OR FULLY GROUTED UNITS, FOR SLABS OR STEEL DECK BEARING ON MASONRY, UNLESS MORE IS SHOWN ON THE DRAWINGS.

(SUCH AS BEAMS, LINTELS, AND JOISTS) SHALL HAVE VOIDS FILLED WITH 20MPa GROUT FOR A MINIMUM DEPTH OF 400mm (16") OR 3 TIMES THE LENGTH OF BEARING (WHICHEVER IS GREATER) AND PROJECTING A MINIMUM OF 200mm (8") OR THE LENGTH OF BEARING BEYOND EACH EDGE OF BEARING (WHICHEVER IS GREATER), UNLESS OTHERWISE NOTED OR SHOWN.

11. ALL MASONRY BENEATH CONCENTRATED LOADS

12. WHERE STEEL BEARING PLATES ARE SHOWN ON THE DRAWINGS, THEY SHALL BE ANCHORED WITH A MINIMUM OF TWO 12mm DIAMETER x 450mm LONG + 50mm (1/2" DIAMETER x 18" LONG + 2") HOOKED ANCHOR RODS WELDED TO THE PLATES AND EMBEDDED INTO GROUT FILL AS NOTED ABOVE.

13. SEE PLANS AND SCHEDULES REGARDING LINTEL SIZES FOR MASONRY WALLS AND VENEER. FOR ALL OPENINGS OR RECESSES IN MASONRY NOT SHOWN ON DRAWINGS GREATER THAN 300mm (12") AND UP TO 1200mm (4'-0"), INCLUDING THOSE FOR MECHANICAL OR ELECTRICAL SERVICES OR EQUIPMENT, PROVIDE ONE L89x89x6.4 (L3½x3½x¼") ANGLE FOR EACH 100mm (4") THICKNESS OF WALL.

14. MAINTAIN SUPPORT OF MASONRY LINTELS FOR A MINIMUM OF SEVEN DAYS OR UNTIL SUFFICIENT STRENGTH IS GAINED TO SAFELY SUPPORT LOADS

15. FULLY GROUT BLOCK CELLS AT PARAPETS.

16. ALL MASONRY WALLS SHALL BE ADEQUATELY BRACED DURING CONSTRUCTION UNTIL ADEQUATE DIAPHRAGM ACTION CAN BE DEVELOPED BY INSTALLED FLOOR AND ROOF STRUCTURAL COMPONENTS.

17. ALL NON-LOAD BEARING BLOCK WALLS SHALL BE BRACED TO THE STRUCTURE ABOVE AT A MAXIMUM SPACING OF 1800mm (6'-0") O.C.

18. REFER TO ARCHITECTURAL DRAWINGS FOR LOCATIONS OF MASONRY CONTROL JOINTS. SPACING OF CONTROL JOINTS IN ALL WALLS SHALL BE CONSTRUCTED AS PER PLAN, BUT SHALL NOT EXCEED 6000mm (20'-0") O.C ALL REINFORCING TO BE DISCONTINUOUS AT CONTROL JOINTS. CONTROL JOINTS SHALL BE CAULKED WITH FOAM BACKER ROD AND SHALL NOT BE FILLED WITH MORTAR.

19. REINFORCED MASONRY: 19.1. CELLS TO BE REINFORCED SHALL BE KEPT CLEAN

19.2. GROUT FOR REINFORCED CELLS, BOND BEAMS. LINTELS AND CELLS CONTAINING DOWELS, ANCHOR

BOLTS AND INSERTS 19.3. PROVIDE MINIMUM 2-15M VERTICALS FULL HEIGHT AT ALL WALL ENDS, CORNERS, INTERSECTIONS AND OPENINGS UNLESS OTHERWISE NOTED ON

19.4. PROVIDE 1-15M VERTICAL FULL HEIGHT EACH SIDE OF CONTROL JOINTS. 19.5. DOWELS FROM FOUNDATIONS TO MATCH VERTICAL REINFORCEMENT IN WALL.

19.6. PROVIDE THE FOLLOWING LAPS FOR THE REINFORCEMENT INDICATED: - 10M BARS = 450mm (18")-15M BARS = 600mm (24")

- 20M BARS = 900mm (36")

HORIZONTALLY WITHIN WALL.

EMBEDDED ITEMS ARE NOT TO INTERFERE WITH THE INTEGRITY OF THE MASONRY WALL OR LOCATION OF REINFORCEMENT. PROVIDE FULLY GROUTED LINTEL BEAM FOR CONDUITS AND PIPES RUNNING

STRUCTURAL STEEL

1. ALL STRUCTURAL STEEL AND CONNECTIONS SHALL BE 1. CONTRACTOR SHALL PROTECT THE EXISTING 1913 FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST CSA STANDARD S16

STRUCTURAL STEEL SHALL CONFORM TO CAN/CSA-G40.20 FOR GENERAL REQUIREMENTS, AND CAN/CSA-G40.21 FOR QUALITY GRADE 350W CLASS C FOR H.S.S.

MINIMUM GRADE 300W (U.N.O.)

2.2. GRADE 350W FOR W SHAPES, S SHAPES, AND 2.3. ALL OTHER MISCELLANEOUS METAL SHALL BE

BOLTED CONNECTIONS SHALL USE MINIMUM 19mm (¾") DIAMETER ASTM A325 BOLTS. ALL BOLTS, NUTS AND WASHERS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A325 EXCEPT THAT ANCHOR BOLTS SHALL BE FABRICATED FROM STEEL ROD CONFORMING TO CSA STANDARD G40.21 OR ASTM F1554 WITH A MINIMUM YIELD STRENGTH OF 250MPA.

4. STEEL COATINGS - UNLESS NOTED OTHERWISE ALL STRUCTURAL STEEL SHALL BE CLEANED AND PREPARED TO A MINIMUM LEVEL OF SSPC SP-3 AND IN ACCORDANCE WITH CSA STANDARD S16: 4.1. ALL INTERIOR STEEL THAT IS TO BE PROTECTED

BY A SPRAY APPLIED CEMENTITIOUS FIRE

PROOFING SHALL BE CLEANED AND REMAIN UNCOATED STEE 4.2. ALL OTHER INTERIOR STRUCTURAL STEEL SHALL BE SHOP PRIME PAINTED AS PER CSA/CAN-S-16. SHOP PRIMER SHALL CONFORM TO CISC/CPMA

4.3. ALL STEEL EXPOSED TO WEATHER IS TO BE HOT DIP GALVANIZED IN ACCORDANCE TO CAN/CSA-G164. TOUCH UP OF WELDS, CUTS OR SCRATCHES TO GALVANIZING SHALL BE DONE WITH A MINIMUM OF 3 COATS OF ZINC RICH PAINT.

WELDING OF STRUCTURAL STEEL SHALL CONFORM TO THE REQUIREMENTS OF CSA STANDARD W59 AND SHALL BE UNDERTAKEN BY A FABRICATOR AND ERECTOR FULLY APPROVED BY THE CANADIAN WELDING BUREAU TO THE REQUIREMENTS OF CSA STANDARD W47, DIVISION 1 AND DIVISION 2. FABRICATOR TO SUPPLY CERTIFICATION OF FUSION WELDING, AND WELDING MAY ONLY BE CARRIED OUT IN ACCORDANCE WITH OWNER'S SAFETY REGULATIONS REGARDING WELDING.

6. FABRICATOR SHALL DESIGN CONNECTIONS AND THE LIKE IN ACCORDANCE WITH THE OBC FOR THE FORCES. SHOWN ON THE DRAWINGS. WHERE FORCES ARE NOT NOTED ON THE DRAWINGS, BEAM REACTIONS SHALL BE TAKEN AS ONE-HALF OF THE TOTAL UNIFORMLY DISTRIBUTED FACTORED LOADS NOTED ON THE BEAM LOAD TABLES OF PART FIVE OF CISC'S HANDBOOK OF STEEL CONSTRUCTION, LATEST EDITION, PROVIDED NO POINT LOADS ACT ON THE BEAM. ALL WELDS SHALL BE 5mm $(\frac{3}{6})$ MIN. FILLET. ALL BOLTS SHALL BE MIN. M20 (¾") DIAMETER AND PROVIDE MIN. (2) BOLTS PER

7. SHOP DRAWINGS OF STRUCTURAL STEEL SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW BEFORE FABRICATION.

COLD FORM STEEL FRAMING [BY FABRICATOR]

1. DESIGN COLD FORMED STEEL FRAMING IN CONFORMANCE WITH THE REQUIREMENTS OF CSA S136. DESIGN ALL COLD FORMED STEEL FRAMING MEMBERS FOR THE GRAVITY AND LATERAL LOADINGS INDICATED ON THE DRAWINGS AND IN ACCORDANCE WITH THE OBC.

3. CONFORM TO THE DEFLECTION REQUIREMENTS OF CSA S304.1 FOR STUDS SUPPORTING MASONRY VENEER. 4. SHOP DRAWINGS FOR ALL COLD FORMED STRUCTURAL STEEL FRAMING INCLUDING CONNECTION, BRACING AND

5. SHOP DRAWINGS FOR ALL COLD FORMED STRUCTURAL COVER THAT INFORMATION. STEEL FRAMING SHALL SHOW BOTH DESIGN AND INSTALLATION REQUIREMENTS. RETAIN A LICENSED PROFESSIONAL ENGINEER OF THE PROVINCE OF ONTARIO TO PREPARE, SEAL AND SIGN ALL SHOP DRAWINGS; AND

BRIDGING DETAILS SHALL BE SUBMITTED TO THE

ENGINEER FOR REVIEW BEFORE FABRICATION.

TO PERFORM FIELD REVIEW.

6. STEEL SHALL MEET THE REQUIREMENTS OF ASTM A653 STANDARD SPECIFICATION FOR STEEL SHEET, ZINC RESTORED TO MATCH EXISTING ADJACENT PAVEMENT COATED (GALVANIZED) BY THE HOT-DIP PROCESS, STRUCTURAL (PHYSICAL) QUALITY. STEEL STUDS 18 ga. AND LIGHTER SHALL HAVE MINIMUM YIELD STRENGTH OF 230MPa (33ksi). HEAVIER STUDS SHALL HAVE MINIMUM YIELD STRENGTH OF 345MPa (50ksi).

DEMOLITION

HERITAGE DESIGNATED RUMPEL FELT BUILDING FROM DAMAGE AND VIBRATIONS, AS WELL AS RESPECT AND FULFILL ALL REQUIREMENTS REGARDING THE DISPOSAL OF CONTAMINATED SOIL AND GROUNDWATER, BEING AWARE OF HAZARDOUS MATERIALS AS PER THE DESIGNATED SUBSTANCE ASSESSMENT REPORT, AND NOT UNDERMINING EXISTING FOUNDATIONS TO REMAIN. CONTRACTOR TO ENSURE CONTINUOUS SECURITY OF THE BUILDING UNTIL TURN OVER TO THE REGION AND ENSURE HEALTH AND SAFETY CONSIDERATIONS OF ALL WORKERS.

THE CONTRACTOR IS RESPONSIBLE TO MAINTAIN SAFE ACCESS TO PEDESTRIAN AND VEHICLE TRAFFIC. IF UTILITY SERVICES MUST BE INTERRUPTED, THE CONTRACTOR SHALL COORDINATE THAT SHUTDOWN AT LEAST ONE WEEK IN ADVANCE WITH OWNER'S REPRESENTATIVE AND REGULATORY AUTHORITY. LANE REDUCTIONS ARE NOT PERMITTED ON VICTORIA STREET DURING MORNING AND EVENING WEEKDAY RUSH HOURS.

3. CONTRACTOR TO MAINTAIN SAFE ACCESS TO THE EXISTING ADJACENT PUBLICLY ACCESSIBLE AREAS SURROUNDING THE SITE, INCLUDING PEDESTRIAN WALKWAYS, SIDEWALKS AND VEHICULAR ACCESS TO ROADWAYS.

4. EROSION CONTROL SHALL BE IN PLACE PRIOR TO ANY SOIL DISTURBANCE, INCLUDING PAVEMENT REMOVAL

ALL FOUNDATIONS, SLABS, STRUCTURAL STEEL, MASONRY, SIDEWALKS, RETAINING WALLS, CURBS, APPARATUSES, ETC., WITHIN THE DESIGNATED DEMOLITION LINES SHALL BE DEMOLISHED AND REMOVED. ALL EXISTING UTILITIES. PAVEMENT, CURBS, STRUCTURES OR OTHER IMPROVEMENTS MUST BE REMOVED WITHIN PROPOSED DEMOLITION FOOTPRINT.

6. ALL SIDEWALKS, SLABS, FOUNDATIONS AND MISCELLANEOUS DEMOLITION SHALL BE SPOILED OFF-SITE UNLESS OTHERWISE DIRECTED BY THE OWNER'S REPRESENTATIVE. NO BURNING OF DEBRIS SHALL B ALLOWED. REPRESENTATIVE. NO GARBAGE, ORGANICS OR OTHER DEBRIS IS ALLOWED AS FILL FILL PLACED IN LIFTS GREATER THAN 600mm (2'-0") BELOW SUB-GRADE SHALL NOT EXCEED 150mm (6") IN ANY DIMENSION. FILL PLACED IN LIFTS FROM 600mm (2'-0") BELOW SUB-GRADE TO SUB-GRADE SHALL NOT EXCEED 50mm (2") ANY DIMENSION.

THE CONTRACTOR SHALL BE PERMITTED TO SALVAGE ANY EQUIPMENT OR MATERIALS THEY DEEM FEASIBLE FOR THAT PURPOSE. OWNER IS ALLOWED FIRST RIGHT OF REFUSAL. ALL SALVAGED MATERIAL OR ITEMS SHALL BE REMOVED FROM THE SITE IMMEDIATELY UPON REMOVAL OTHERWISE DIRECTED BY THE OWNER'S REPRESENTATIVE. NO SUCH MATERIALS SHALL BE STORED ON THE SITE. ABSOLUTELY NO SALES OF SALVAGED MATERIALS WILL BE ALLOWED ON THE PROJECT SITE. ANY SALVAGED MATERIAL MUST BE REMOVED AND TRANSPORTED IN A LEGAL MANNER.

8. ALL EXISTING ON-SITE UTILITIES SHALL REMAIN UNLESS DESIGNATED FOR REMOVAL OR SHOULD THEY INTERFERE WITH PROJECT CONSTRUCTION. CONTRACTOR SHALL PROTECT ALL EXISTING UTILITIES TO REMAIN. ALL UTILITIES WITHIN THE BUILDING FOOTPRINT WILL BE REMOVED UNLESS OTHERWISE NOTED.

9. MANHOLES, CATCH BASINS, CLEAN OUTS, VALVE BOXES, FRAMES, COVERS AND GRATES REMAINING IN USE SHALL BE PROTECTED AND ADJUSTED TO FINAL GRADES. 10. ALL UTILITIES SHOWN TO BE REMOVED SHALL BE

DISPOSED OF OFF-SITE IN A LEGAL MANNER.

REMOVE EXISTING UTILITIES. IT IS THE CONTRACTOR'S RESPONSIBILITY TO PROVIDE PROPER REMOVAL. INCLUDING SAFE SEQUENCING OF REMOVAL FOR ALL UTILITIES. CONTRACTOR SHALL COORDINATE UTILITY REMOVALS WITH UTILITY PROVIDERS AND WITH CITY OF KLICHENER AND ANY APPLICABLE GOVERNIN AUTHORITIES. MAINTAIN POSITIVE DRAINAGE AT ALI TIMES. DRAINAGE CONTROLS ARE REQUIRED, NO OFF SITE DRAINAGE IS PERMITTED. REFER TO MTE'S SOIL AND GROUNDWATER MANAGEMENT PLAN AND ADD NOTES TO

12. FOR ALL UTILITY LINES AND STRUCTURES DESIGNATED TO BE REMOVED. PLACE AND COMPACT STRUCTURAL BACKFILL WITHIN TRENCH IN LIFTS OF MAXIMUM DEPTH OF 12" COMPACTED TO 95% SPMDD. ANY PAVEMENT STRUCTURES THAT ARE REMOVED AS PART OF THE UTILITY REMOVALS/CAPPING SHALL BE REPLACED AND

13. CONTRACTOR IS RESPONSIBLE TO VERIEY GRADES. AND UTILITIES SHOWN ON EXISTING CONDITIONS PLAN PRIOR TO START OF ANY WORK. ANY AND ALL DISCREPANCIES ARE TO BE DOCUMENTED AND SUBMITTED TO THE OWNER'S REPRESENTATIVE AND THE CONSULTANT AT THE TIME OF DISCOVERY.

14. CONTRACTOR SHALL COORDINATE ALL DEMOLITION WORK WITH APPROPRIATE UTILITY COMPANIES PRIOR TO STARTING WORK.

15. A PHASE II ESA HAS ALREADY BEEN CONDUCTED, AS WELL AS A DESIGNATED SUBSTANCE SURVEY. CONTRACTOR SHALL READ THESE REPORTS AND COMPLY WITH ALL APPLICABLE REGULATIONS. THE BUILDINGS HAVE DESIGNATED SUBSTANCES, AND THE SITE HAS CONTAMINATED SOIL AND GROUNDWATER.

16. ALL DEBRIS AND SOILS, DERIVED FROM THE CONTRACTOR'S OPERATIONS, FOUND IN THE PUBLIC RIGHT-OF-WAY OR CAUSING NUISANCE TO OPERATIONS, SHALL BE CLEANED AND REMOVED ON A DAILY BASIS OR WHEN NOTIFIED BY THE AUTHORITY HAVING JURISDICTION OR THE OWNER'S REPRESENTATIVE.

| | DRAWING LIST |
|------|---|
| NO. | NAME |
| A1.0 | CONSTRUCTION NOTES |
| A2.0 | WORK PLAN |
| A2.1 | DEMOLITION/REMEDIATION FLOOR PLAN ONE |
| A2.2 | DEMOLITION/REMEDIATION FLOOR PLAN TWO |
| A2.3 | DEMOLITION/REMEDIATION FLOOR PLAN THREE |
| A2.4 | DEMOLITION/REMEDIATION ROOF PLAN |
| A3.1 | ELEVATIONS |
| A3.2 | ELEVATIONS |
| A4.0 | DEMOLITION/REMEDIATION DETAILS |
| A5.0 | PHOTO DETAILS |
| S2.1 | DEMOLITION/REMEDIATION FOUNDATION PLAN |
| S2.2 | DEMOLITION/REMEDIATION FLOOR TWO FRAMING PLAN |
| S2.3 | DEMOLITION/REMEDIATION FLOOR THREE FRAMING PLAN |
| S2.4 | DEMOLITION/REMEDIATION ROOF FRAMING PLAN |
| S3.1 | DEMOLITION and STABILIZATION FLOOR TWO FRAMING PLAN |
| S3.2 | DEMOLITION and STABILIZATION FLOOR THREE FRAMING PLAN |
| S3.3 | DEMOLITION and STABILIZATION ROOF FRAMING PLAN |
| C1.1 | EXISTING CONDITIONS PLAN |
| C2.1 | DEMOLITION AND EROSION & SEDIMENT CONTROL PLAN |

CONTRACTOR SHALL READ AND COMPLY WITH THE HERITAGE CONSERVATION PLAN (HCP)

RISK MANAGEMENT PLAN
THE DEMOLITION AND STABILIZATION PLAN HAS BEEN DEVELOPED WITH THE INTENT TO

IF REQUIRED, EARTH RETENTION SHORING SHALL BE DESIGN USING DRILLED OR AUGURED

IF REQUIRED, TEMPORARY BUILDING SHORING SHALL UTILIZE HELICAL SCREW PILES SUCH

CONTINUOUS VIBRATION MONITORING OF THE 1913 BUILDING WILL BE IMPLEMENTED BY THE CONTRACTOR AND THE HERITAGE PROFESSIONAL SHALL REGULARLY VISIT THE SITE

DURING DEMOLITION TO REVIEW THE FAÇADE OF THE 1913 BUILDING FOR ANY IMPACT,

IF DAMAGE TO THE HERITAGE RESOURCE DOES OCCUR DURING DEMOLITION. WORK IN

CONTRACTOR HAS ASSESSED THE CAUSE FOR THE DAMAGE AND DEVELOPED UPDATED

SALVAGED BRICKS SHALL BE REMOVED AND CLEANED BY HAND PROCESSES TO PREVENT

DAMAGE. BRICKS SHALL BE STORED ON PALLETS, PROTECTIVELY WRAPPED AND STORED

THAT AREA OF THE STRUCTURE SHALL IMMEDIATELY BE STOPPED. THE HERITAGE

PROFESSIONAL SHALL BE CALLED IN TO REVIEW THE DAMAGE AND DEVELOP A

RESTORATION PLAN. WORK SHALL NOT CONTINUE IN THAT AREA UNTIL THE

MINIMIZE VIBRATION AND OTHER CONSTRUCTION ACTIVITIES TO THE PROPERTY DURING

PILES SUCH THAT PILE DRIVING IS NOT REQUIRED FOR THE REMOVAL OF THE

THE CONTRACTOR SHALL REGULARLY MONITOR THE REMAINING FAÇADE DURING

DEMOLITION TO ENSURE NO DEGRADATION OF THE FAÇADE IS OCCURRING.

DEMOLITION. THE MITIGATION MEANS ARE NOTED BELOW.

NO VIBRATORY EQUIPMENT SHALL BE USED ON SITE.

PROCEDURES TO PREVENT IT FROM HAPPENING FURTHER.

FOUNDATIONS OF THE BUILDING ADDITIONS.

THAT PILE DRIVING IS NOT REQUIRED.

SHIFTING OR NEW DETERIORATION.

IN A SAFE LOCATION.

AND THE RISK MANAGEMENT PLAN (RMP) SUBMITTED TO THE CITY OF KITCHENER.

C2.1 | DEMOLITION AND EROSION & SEDIMENT CONTROL PLAN C2.2 POST DEMOLITION PLAN

> **ISSUED FOR** HERITAGE REVIEW SECOND DRAFT **APR 08, 2025**

NOTE TO CONTRACTOR

CONTRACTORS MUST CHECK AND VERIEY ALL DIMENSIONS. AND REPORT ANY DISCREPANCIES TO THE ENGINEER BEFORE

THE OWNER/ARCHITECT/CONTRACTOR IS ADVISED THAT

OF CONSTRUCTION TO ARRANGE FOR INSPECTION.

M.T.F. CONSULTANTS INC. CANNOT CERTIFY ANY COMPONENT

OF THE SITE WORKS NOT INSPECTED DURING CONSTRUCTION

NOTIFY M.T.E. CONSULTANTS INC. PRIOR TO COMMENCEMENT

IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR T

DO NOT SCALE DRAWINGS.

PROCEEDING WITH THE WORK.

HERITAGE REVIEW - SECOND DRAFT 4 APR 08/25 DRAFT REVIEW SET 3 MAR 04/25 RAFT REVIEW SET 2 FEB 28/25 DRAFT REVIEW SET 1 NOV 08/24



519-743-6500

REGIONAL MUNICIPALITY

PROJECT

DEMOLITION

DRAWING

CONSTRUCTION

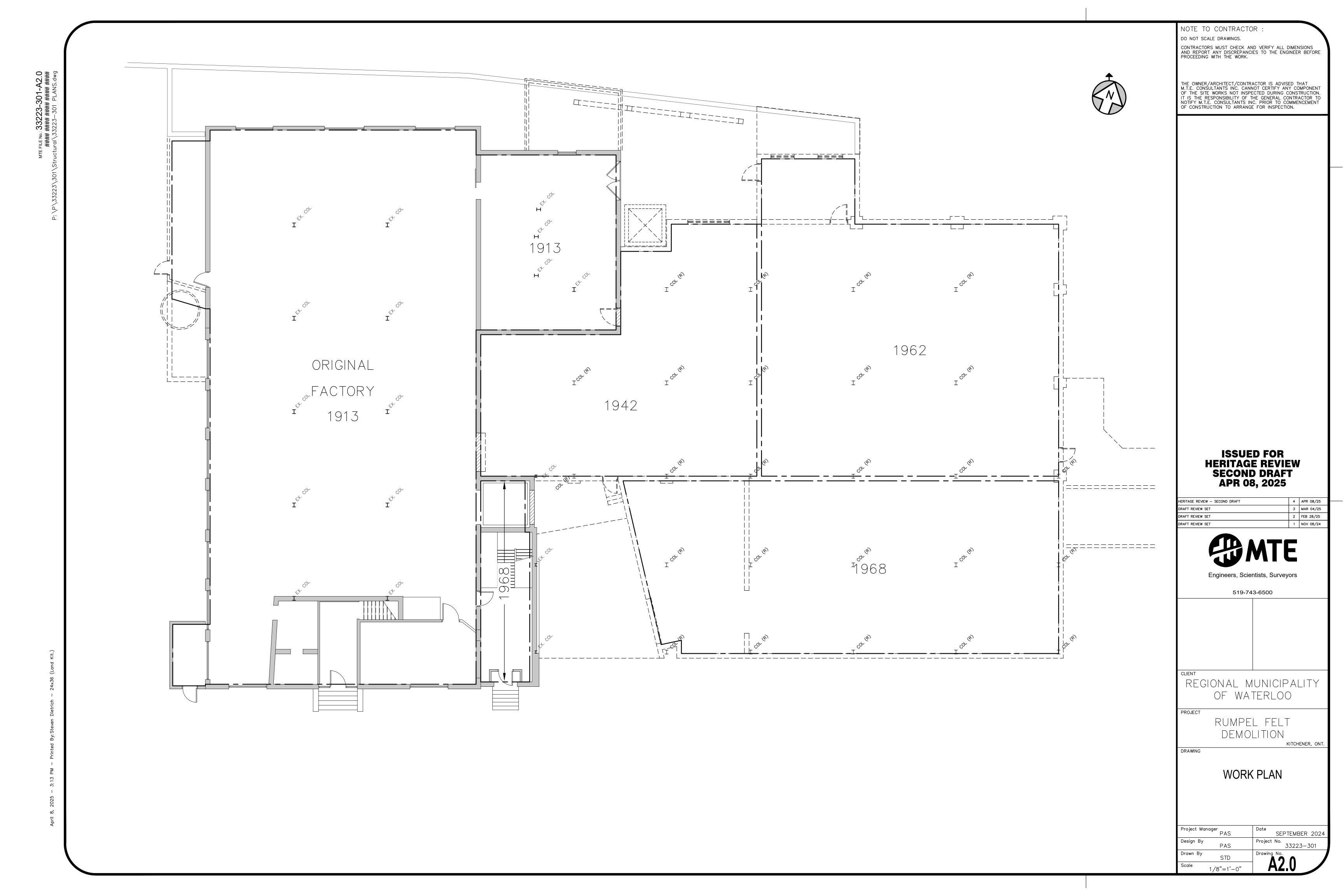
Project Manager PAS SEPTEMBER 2024 Design By Project No. PAS 33223-301 Drawn By Drawina No STD

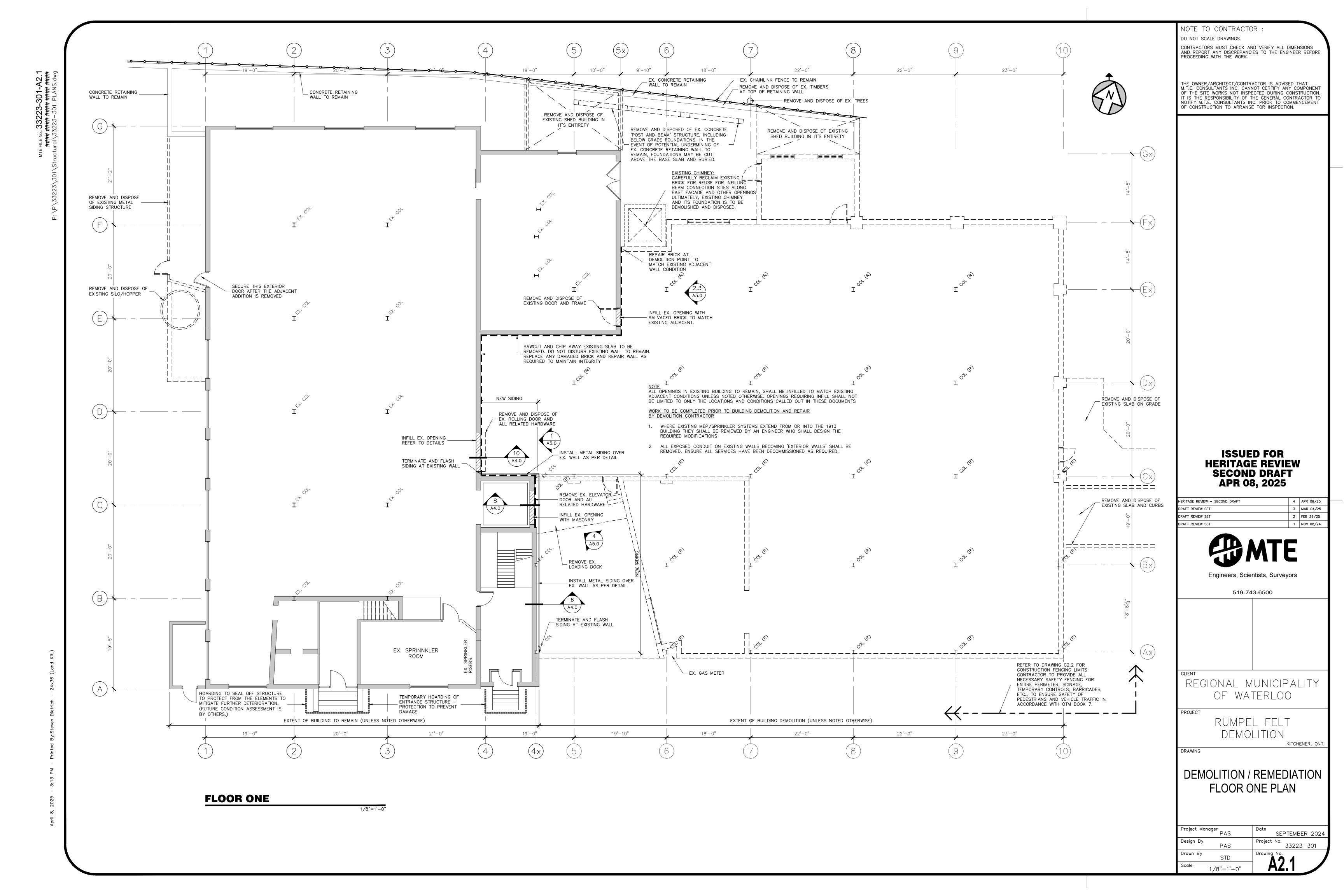
OF WATERLOO

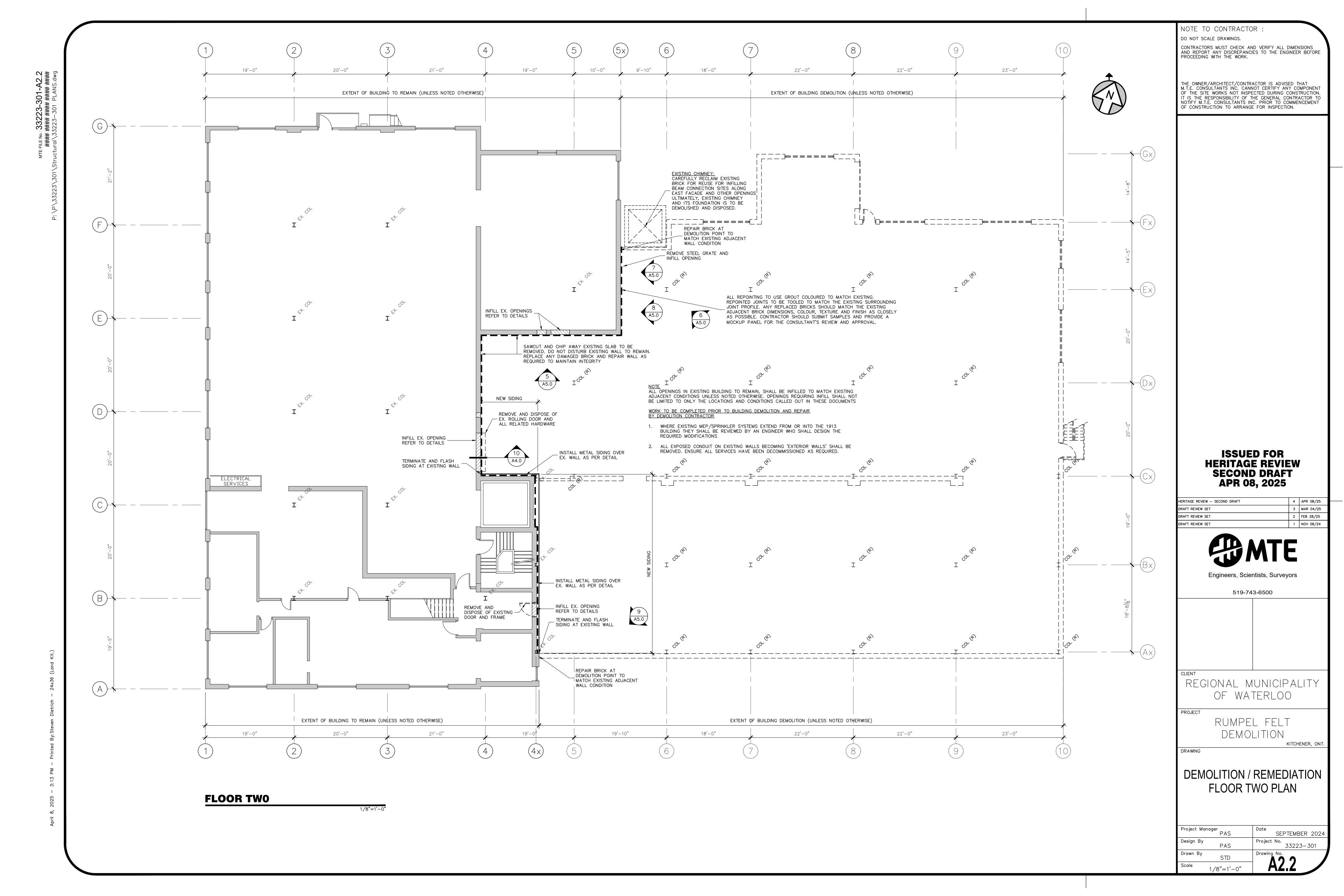
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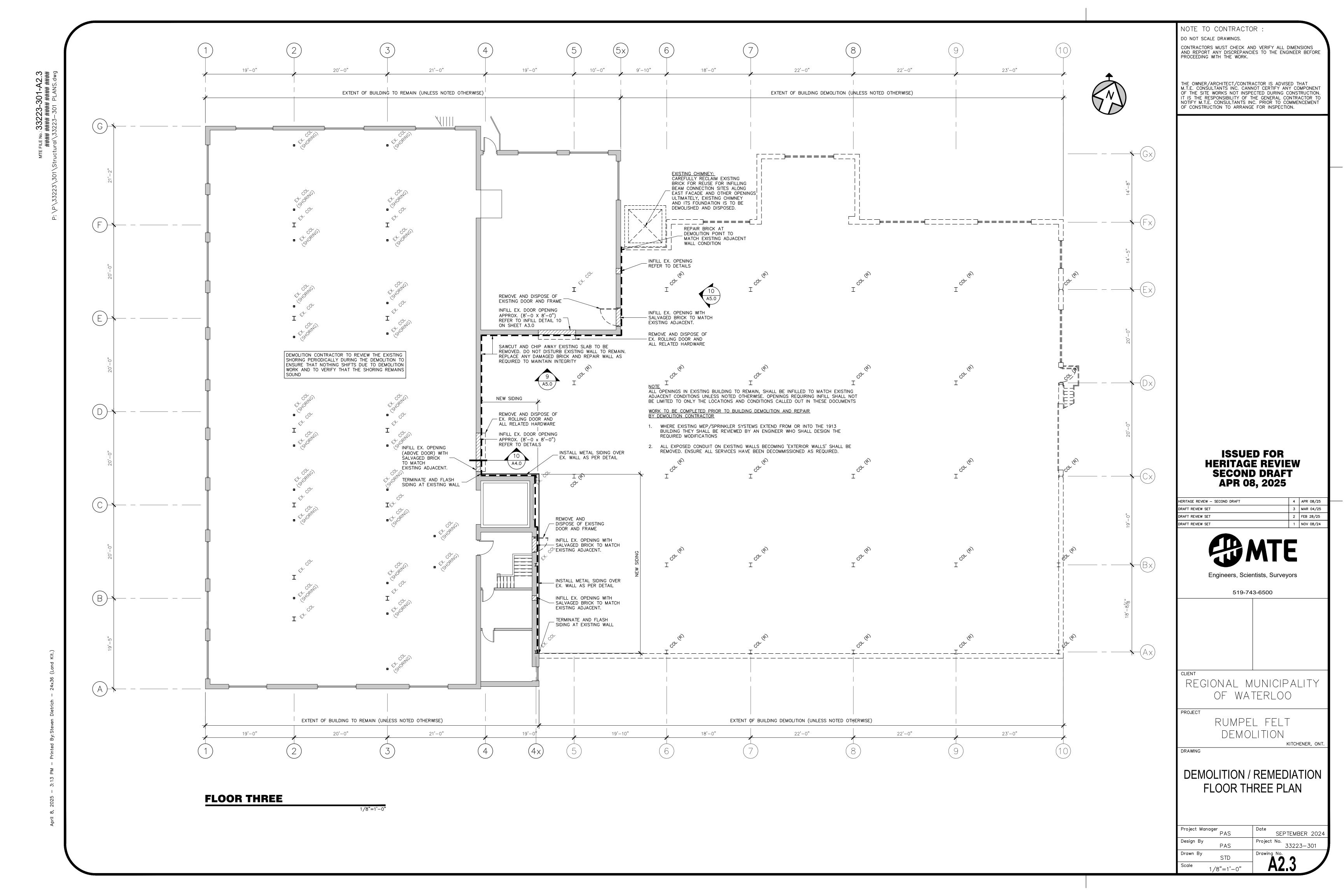
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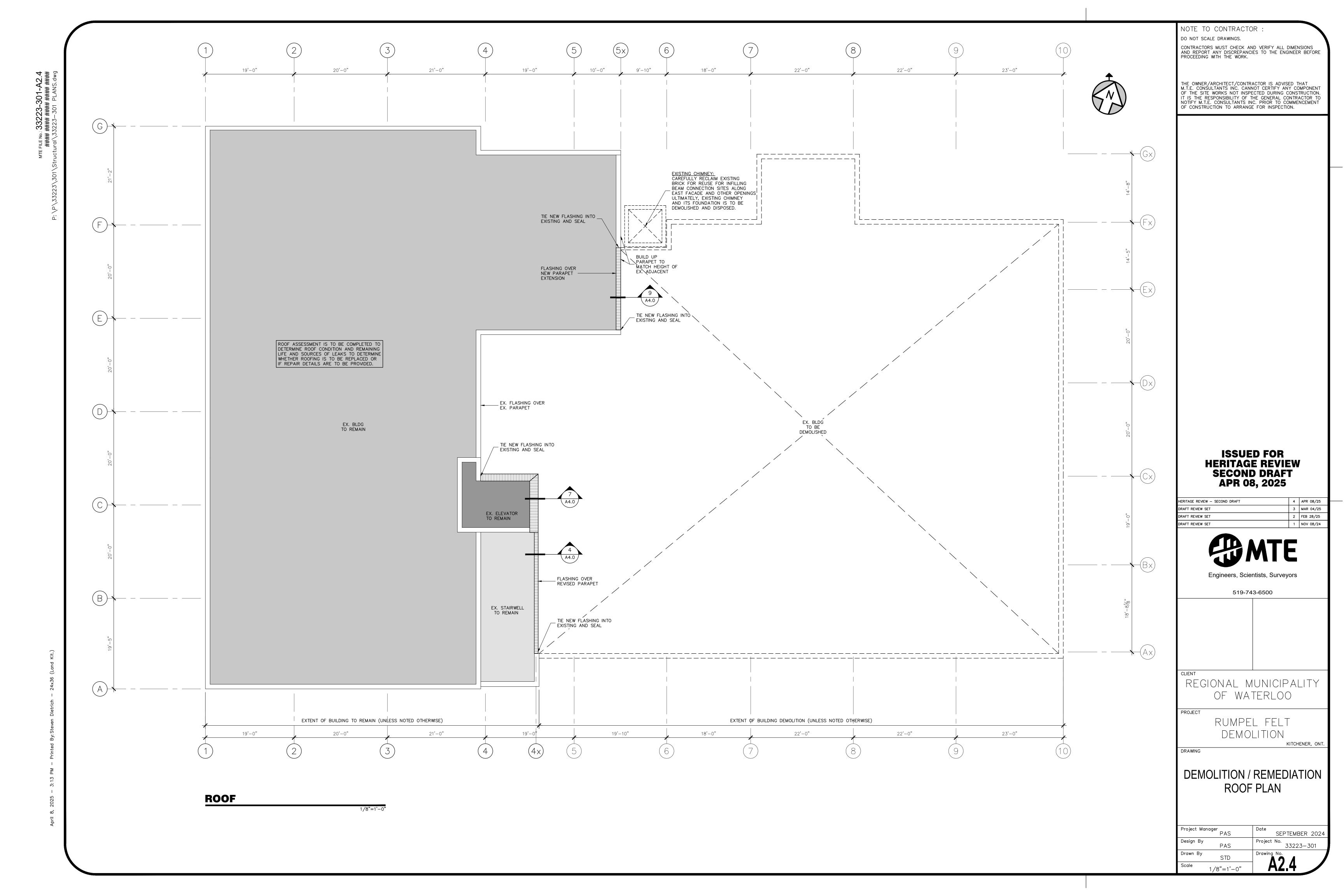
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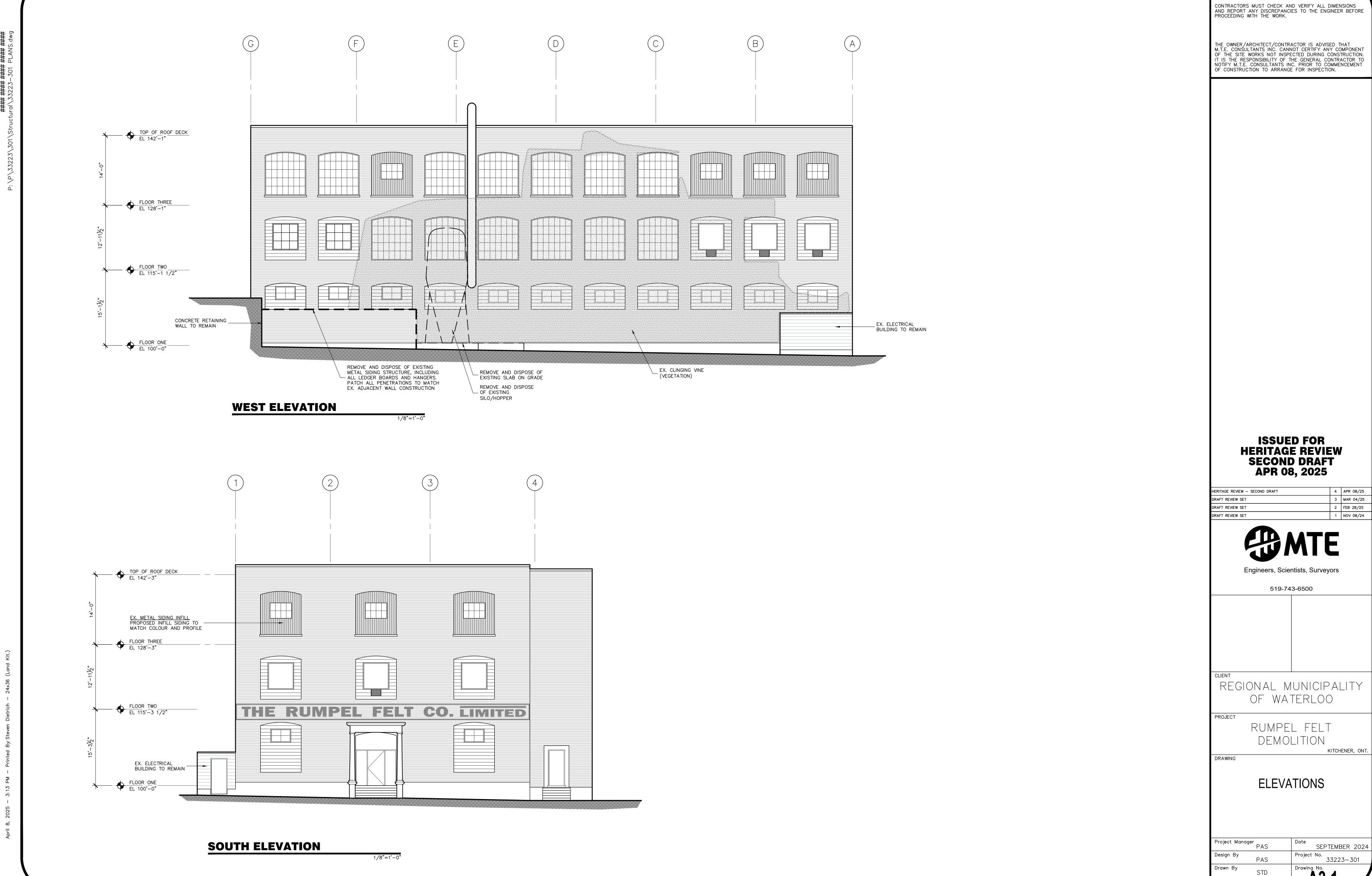












NOTE TO CONTRACTOR

DO NOT SCALE DRAWINGS.

4 APR 08/25 3 MAR 04/25 2 FEB 28/25

FLOOR ONE EL 100'-0"

CONC. FOUNDATION TO

OF EX. ADJACENT

- MATCH HEIGHT AND PROFILE

EX. LOADING RAMP

- AND PLATFORM TO

1/8"=1'-0

BE REMOVED

EX. STRIP FOOTING —

EAST ELEVATION

1/8"=1'-0"

REPAIR BRICK AT DEMOLITION

- POINT TO MATCH EXISTING

ADJACENT WALL CONDITION

EX. PAINTED BRICK WALL (SHADED)

- TO REMAIN. PATCH AND RÈPAIR AS

NORTH ELEVATION

NOTED IN PLANS AND DETAILS

ISSUED FOR HERITAGE REVIEW SECOND DRAFT APR 08, 2025

NOTE TO CONTRACTOR

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THE OWNER/ARCHITECT/CONTRACTOR IS ADVISED THAT M.T.E. CONSULTANTS INC. CANNOT CERTIFY ANY COMPONENT OF THE SITE WORKS NOT INSPECTED DURING CONSTRUCTION.

IT IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO NOTIFY M.T.E. CONSULTANTS INC. PRIOR TO COMMENCEMENT

OF CONSTRUCTION TO ARRANGE FOR INSPECTION.

DO NOT SCALE DRAWINGS.

APR 08, 2025

HERITAGE REVIEW - SECOND DRAFT

DRAFT REVIEW SET

4 APR 08/25

3 MAR 04/25

2 FEB 28/25

KITCHENER, ONT.



519-743-6500

REGIONAL MUNICIPALITY
OF WATERLOO

RUMPEL FELT DEMOLITION

DRAWING

PROJECT

DRAFT REVIEW SET

ELEVATIONS

Project Manager
PAS

Date
SEPTEMBER 2024

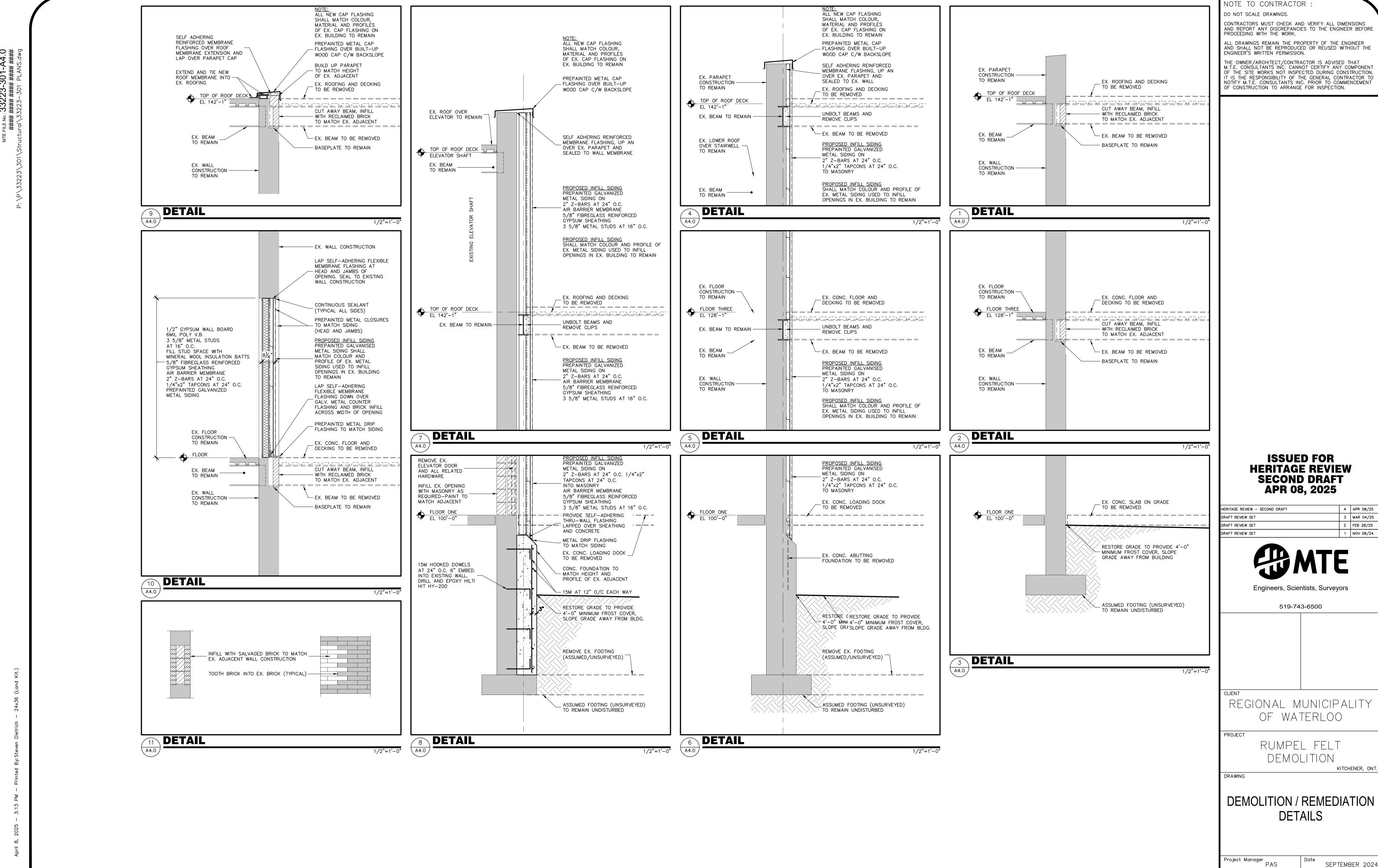
Project No.
33223-301

Design By PAS Project No. 33223-301

Drawn By STD Prawing No. A3.2

| 1/8"=1'-0" | A3.2

April 8, 2025 — 3:13 PM — Printed By: Steven Dietrich



Project No. 33223-301

Design By

Drawn By

Scale

PAS

STD

AS NOTED

SEPTEMBER 2024

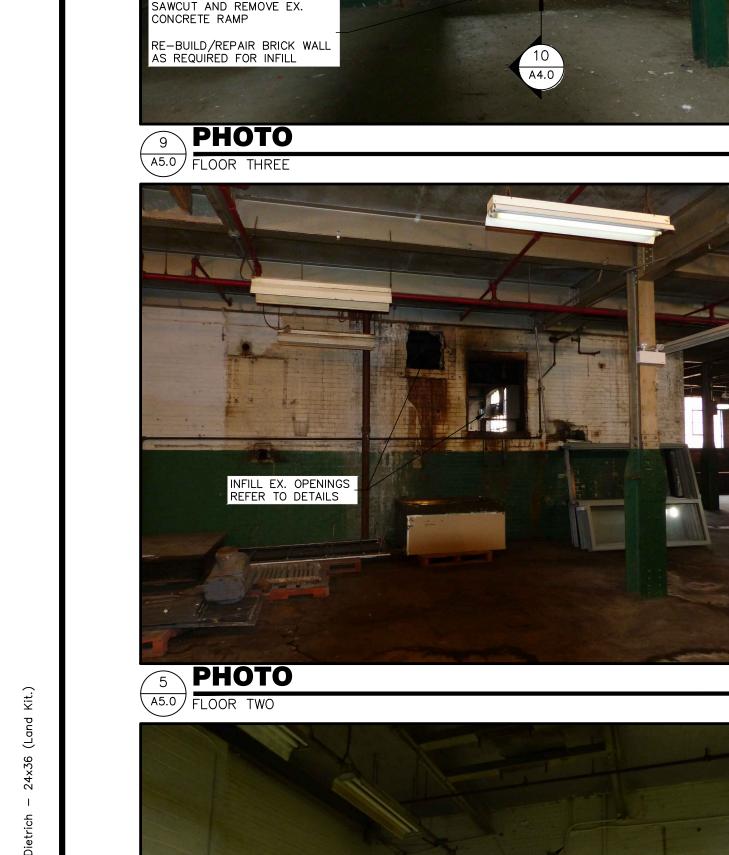
NFILL EX. OPENINGS

REMOVE AND DISPOSE OF

X. ROLLING DOOR AND ALL RELATED HARDWARE

REFER TO DETAILS

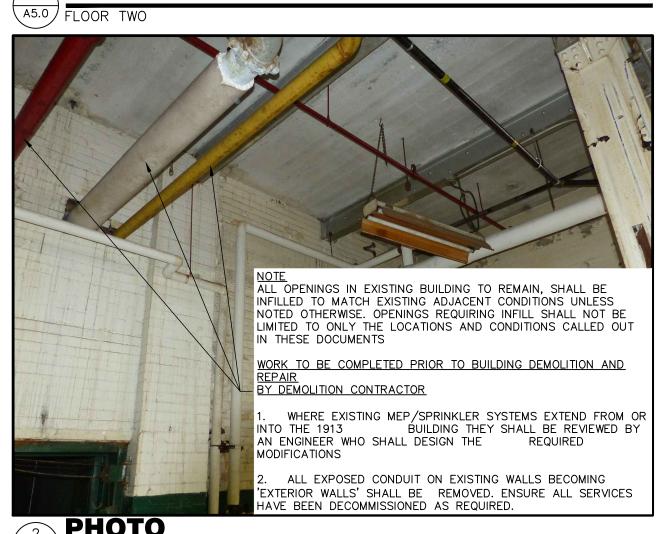
A5.0 FLOOR ONE

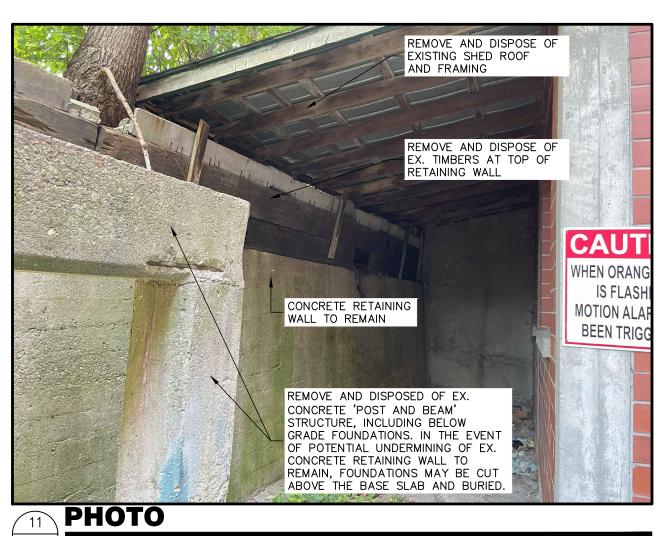




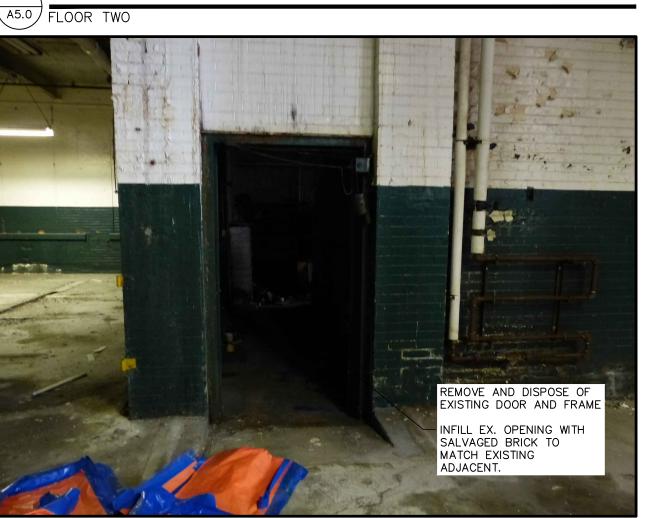


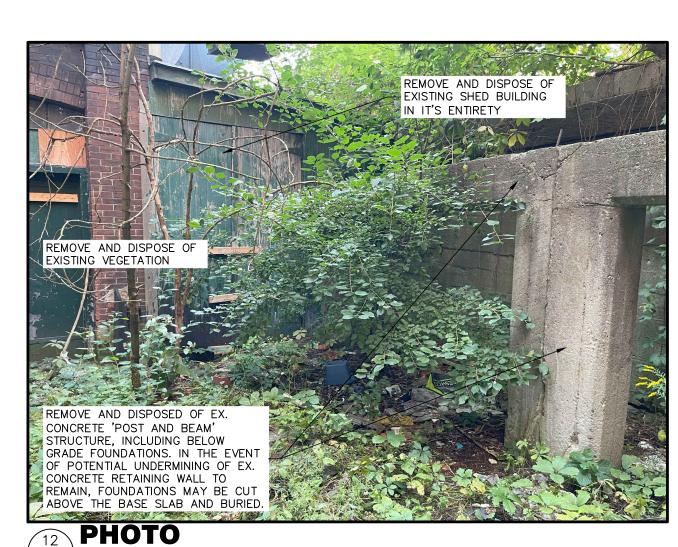
A5.0 FLOOR THREE ALL OPENINGS IN EXISTING BUILDING TO REMAIN, SHALL BE INFILLED TO MATCH EXISTING ADJACENT CONDITIONS UNLESS NOTED OTHERWISE. OPENINGS REQUIRING INFILL SHALL NOT BE LIMITED TO ONLY THE LOCATIONS AND CONDITIONS CALLED OUT IN THESE DOCUMENTS WALL OF EX. CHIMNEY TO BE DEMOLISHED WORK TO BE COMPLETED PRIOR TO BUILDING DEMOLITION AND REPAIR BY DEMOLITION CONTRACTOR 1. WHERE EXISTING MEP/SPRINKLER SYSTEMS EXTEND FROM OR INTO THE 1913 INFILL EX. OPENINGS REFER TO DETAILS BUILDING THEY SHALL BE REVIEWED BY AN ENGINEER WHO SHALL DESIGN THE REQUIRED MODIFICATIONS 2. ALL EXPOSED CONDUIT ON EXISTING WALLS BECOMING 'EXTERIOR WALLS' SHALL BE REMOVED. ENSURE ALL SERVICES HAVE BEEN DECOMMISSIONED AS REQUIRED. INFILL EX. OPENINGS REFER TO DETAILS PHOTO











A5.0 EXTERIOR REPOINT EXISTING BRICK WALL. REPLACE ALL CRACKED/LOOSE BRICK



PHOTO

A5.0 EX. LOADING DOCK

ISSUED FOR HERITAGE REVIEW SECOND DRAFT

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| HERITAGE REVIEW - SECOND DRAFT | 4 | APR 08/ |
|--------------------------------|---|----------|
| DRAFT REVIEW SET | 3 | MAR 04/ |
| DRAFT REVIEW SET | 2 | FEB 28/2 |
| DRAFT REVIEW SET | 1 | NOV 08/ |

Engineers, Scientists, Surveyors

519-743-6500

REGIONAL MUNICIPALITY OF WATERLOO

> RUMPEL FELT DEMOLITION

PROJECT

PHOTO DETAILS

KITCHENER, ONT.

| Project Man | ager | Date |
|-------------|----------|----------------|
| | PAS | SEPTEMBER 2024 |
| Design By | | Project No. |
| | PAS | 33223-301 |
| Drawn By | | Drawing No. |
| - | STD | _ |
| Scale | AC NOTED | ⊢ AD.U ∕ |
| | A2 MOTED | |

PHOTO

A5.0 FLOOR ONE

PHOTO A5.0 FLOOR ONE

